



# Seismic Resistant Design of Connections with the use of Perforated Beams

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In the era of **sustainable** and **resilient** infrastructures, where the concept of **redundancy** plays a significant role, we should reconsider optimising every single structure to the best of its efficiency.

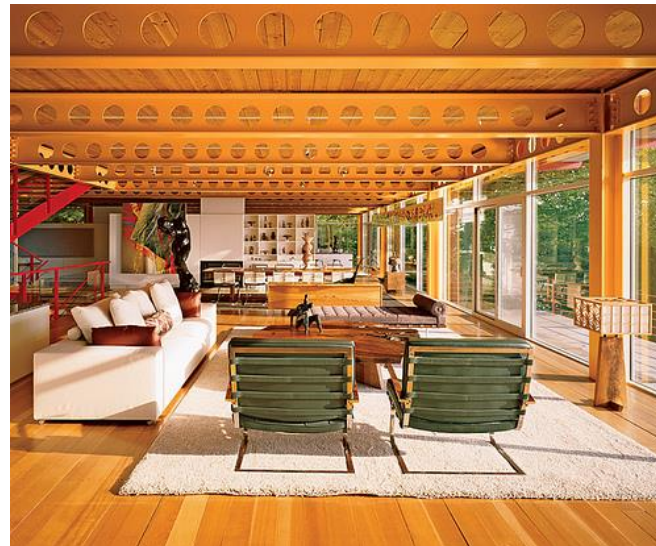
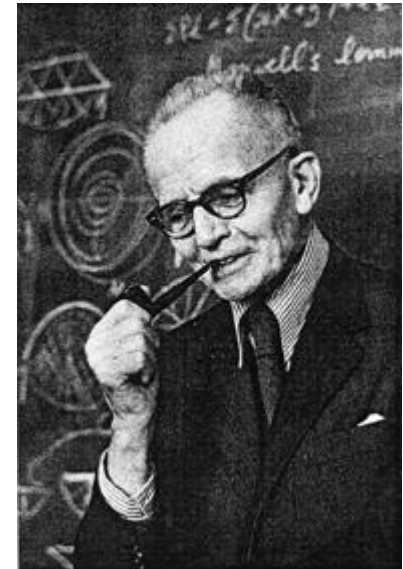
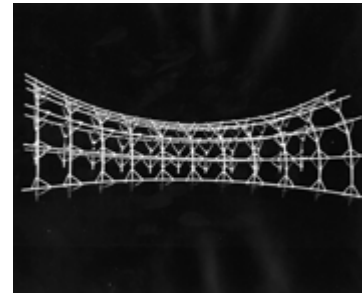
*This can be called as:*

- *Sustainable design*
- *Resilience*
- *Efficiency*
- *Optimisation*
- *Performance-based design*
- *Adaptive (eg. pneumatic) design*

Indeed the **one-off nature** of every civil-structural engineering project necessitates the use of rigorous studies and advanced techniques in the global and local level (as well as the material level) to drive efficiencies on the increasingly complex projects of today.

## *Robert le Ricolais, 1894-1977*

An apparently simple sentence, *The art of structure is where to put the holes*, is his point of departure for an exposition on strength without weight in a tube shape that might create a new kind of subway tunnel.



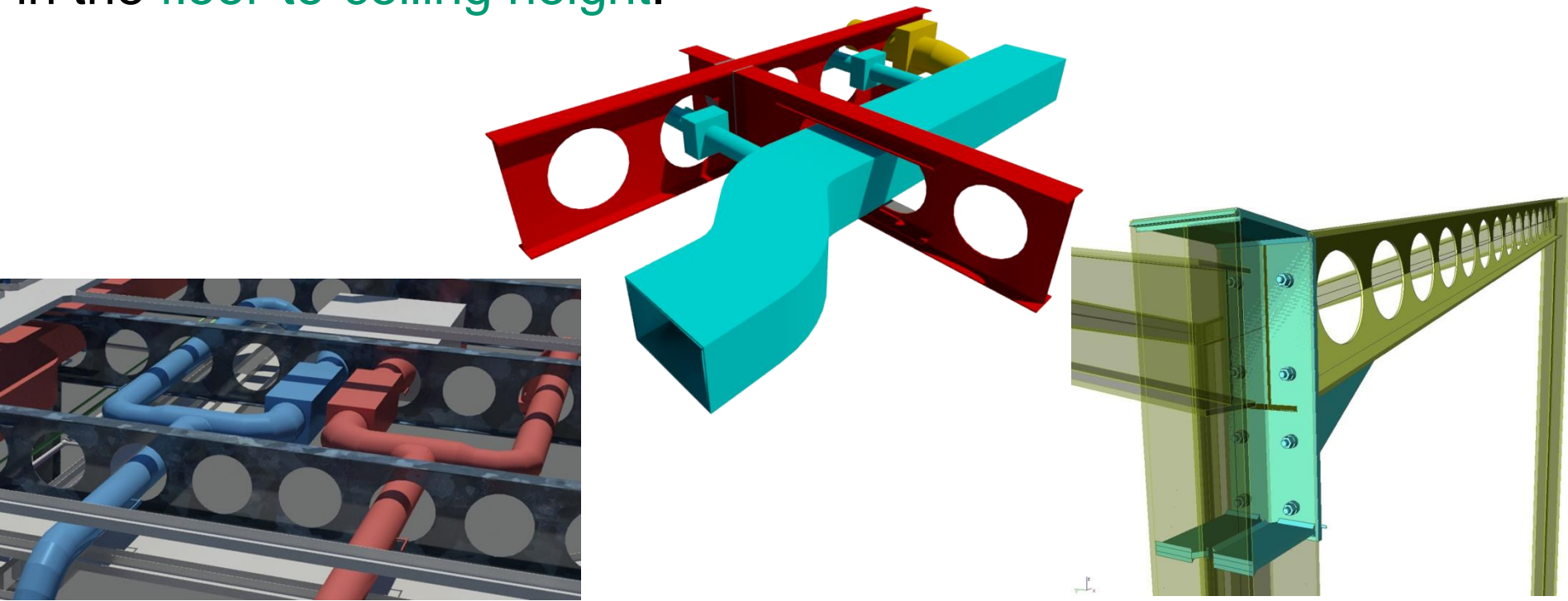
David Hovey,  
Architect

*I have spent my career thinking about how to design buildings economically and efficiently!*



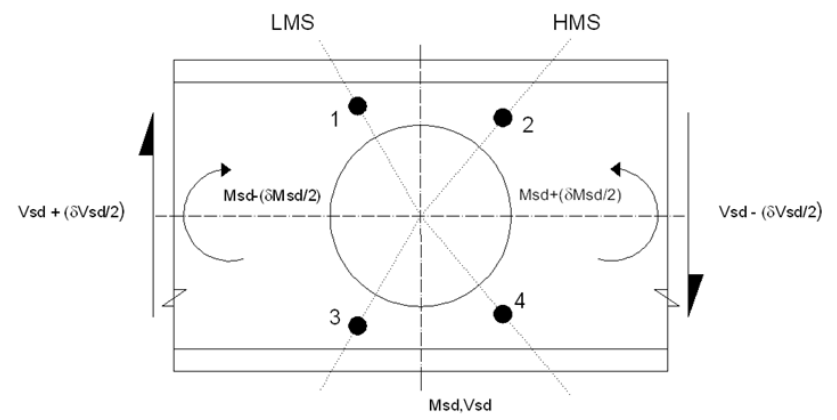
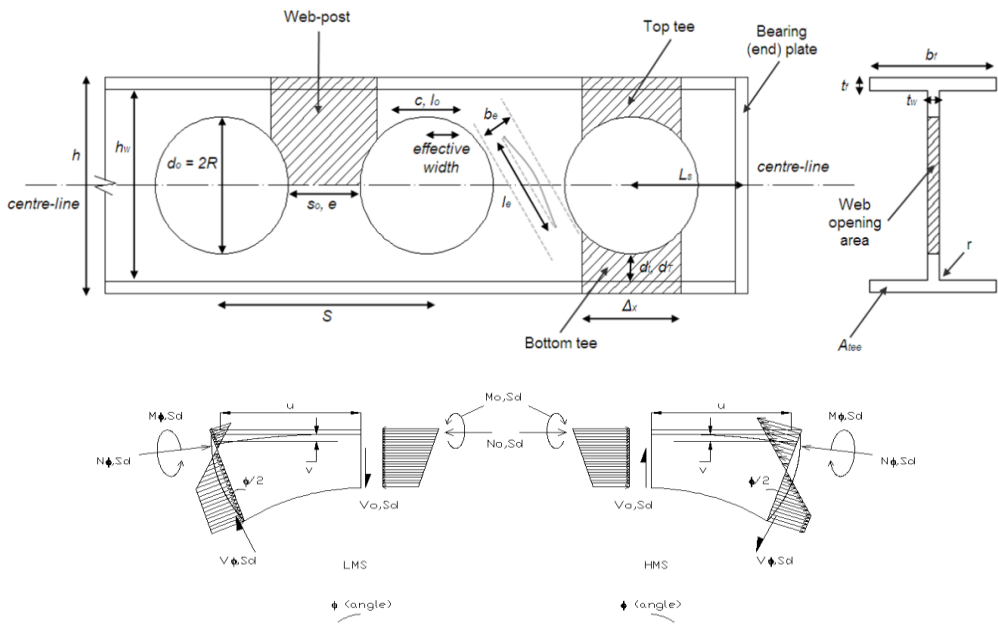
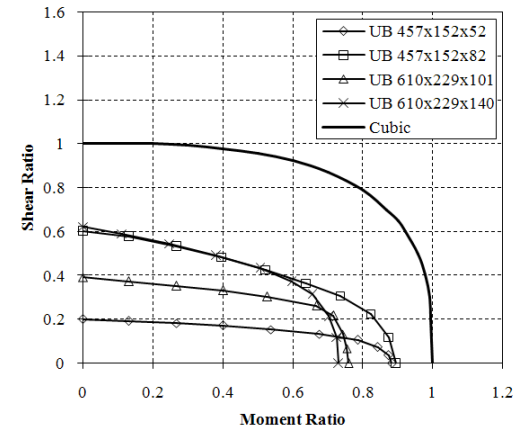
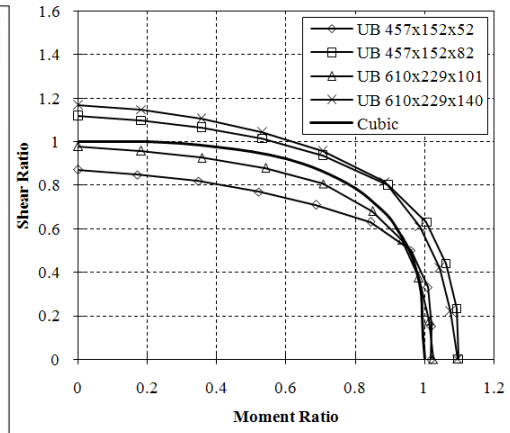
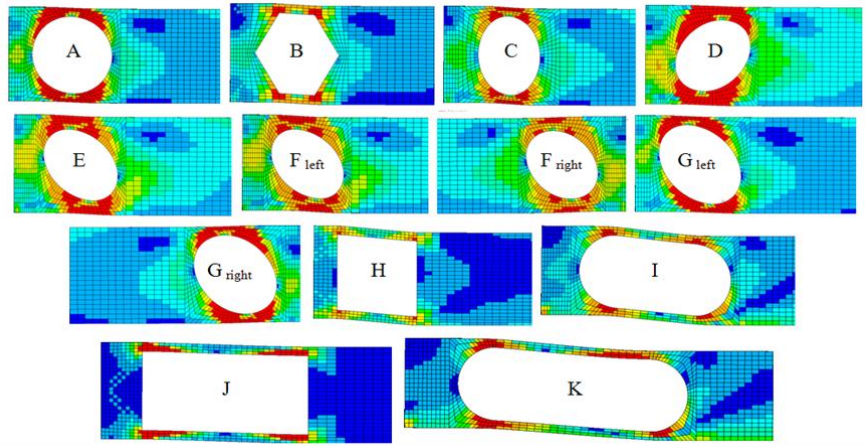
# Perforated Beams

Perforations in the webs of steel beams are widely used nowadays in building construction due to their ability to provide **lighter** structural members, reduced material **costs**, in addition to the provision for greater **flexibility** in structural layouts particularly in the **floor-to-ceiling height**.





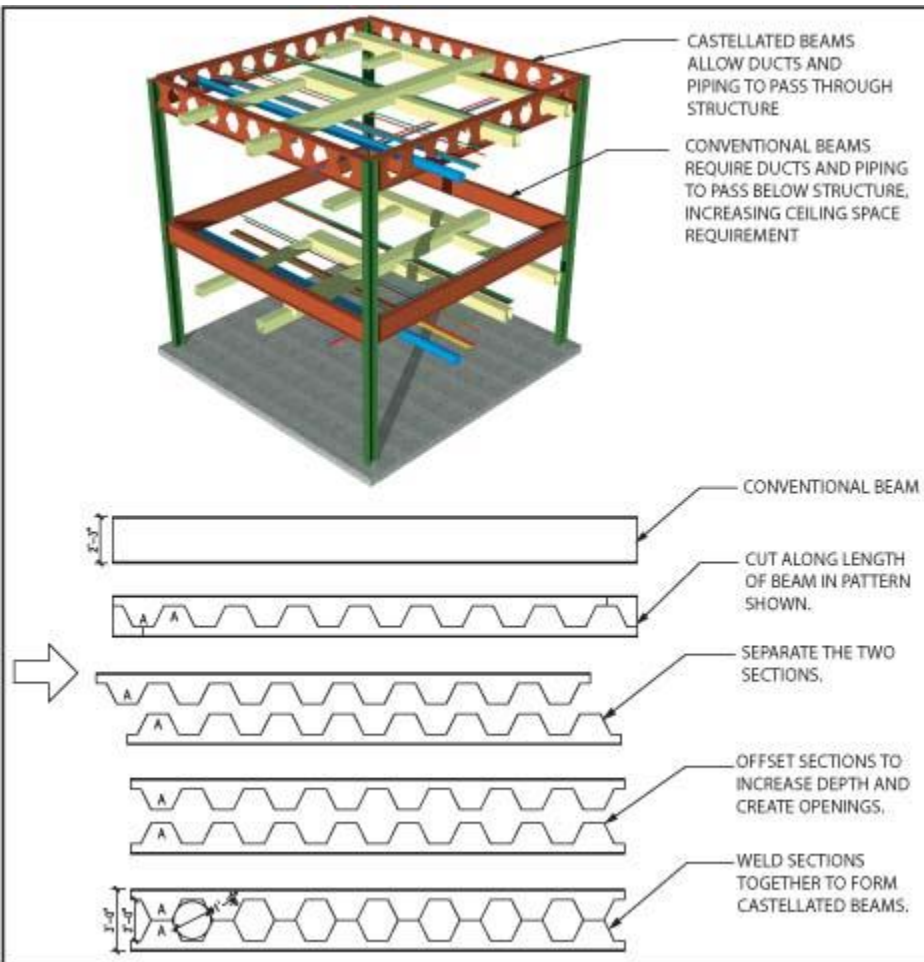
# Perforated Beams (since 2005...)



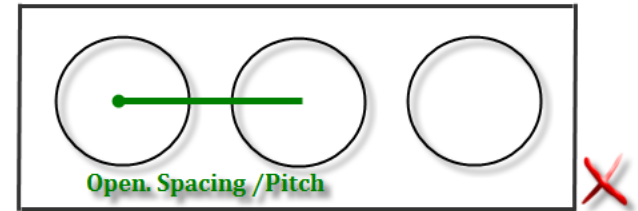




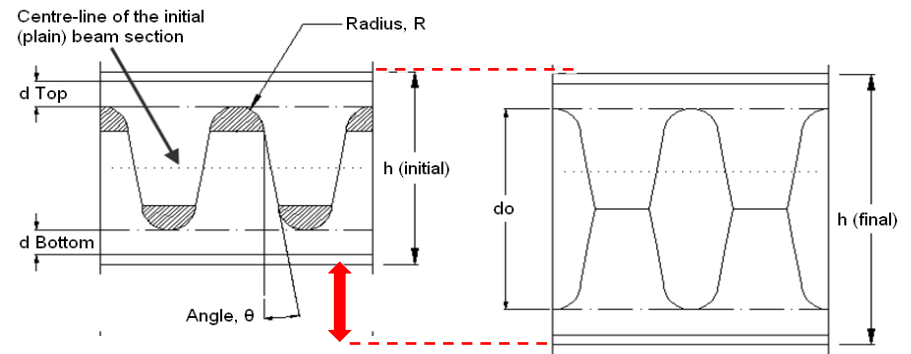
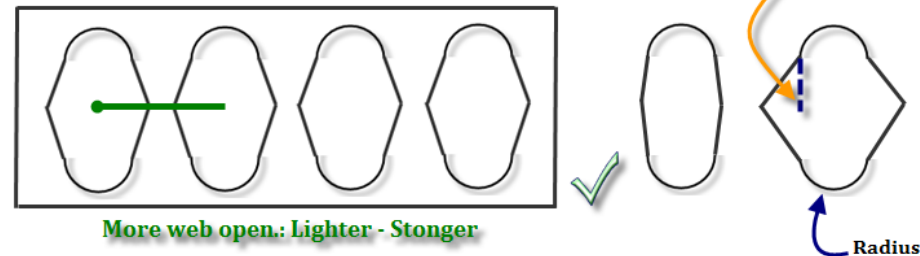
# Perforated Beams (since 2005...)



## CIRCULAR WEB OPENINGS



## NOVEL PATENTED WEB OPENINGS



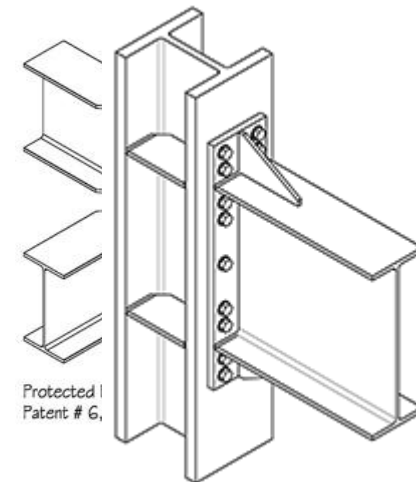
In order to enhance of (post-Northridge) connections, researchers provided **high level of ductility** and assured **operation** in connections by:

➤ Connection **reinforcement/strengthening**; a number of connections have been proposed in FEMA 350 such as the:

- Bolted Flange Plate (BFP)
- Bolted Unstiffened End-Plate (BUEEP)
- Stiffened Extended End-Plate (BSEEP)
- CONXTECH CONX and KAISER Bolted Bracket (KBB)

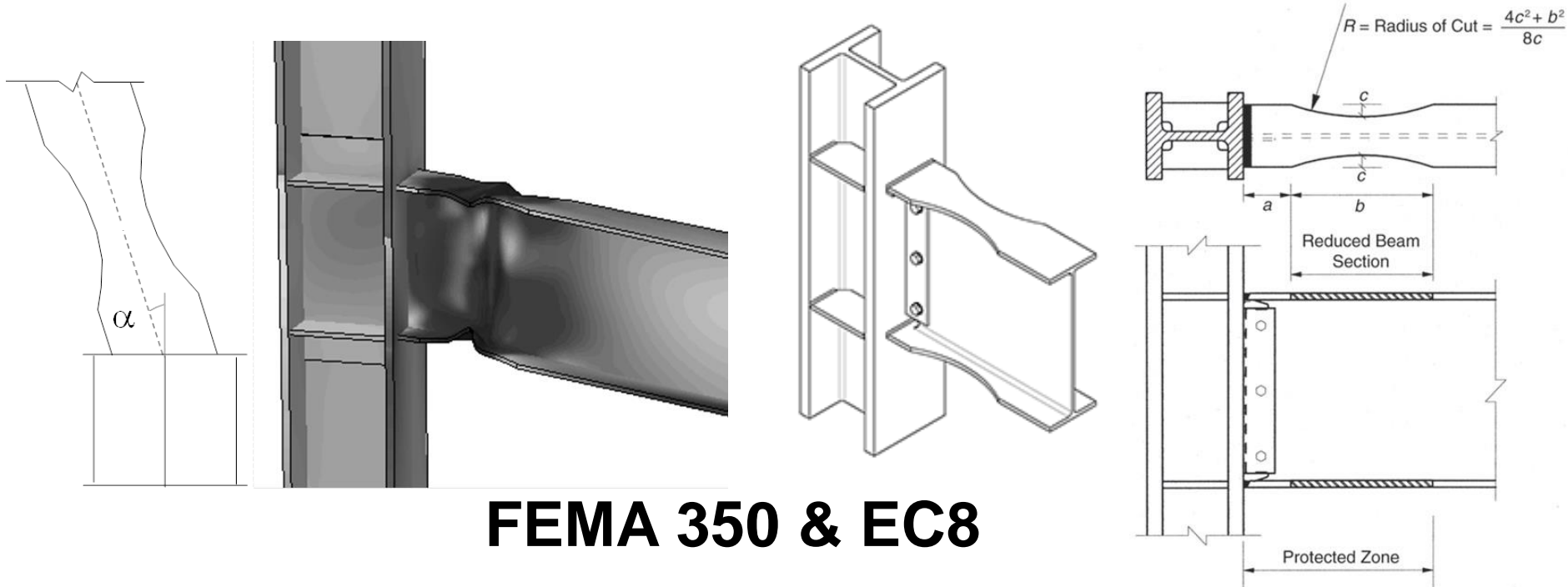
➤ **Excess reinforcement leads to:**

- increasing welding and bolting processes
- increasing connections rigidity and panel zone stresses
- increase in weight and hence the seismic effects



# Weakening Techniques - RBS

➤ Beam weakening by **reducing the cross-sectional area** of the beam locally at a certain distance from the connection to **shift the stresses away from the column's face**. One method to achieve this is by removing portions of the flanges locally, referred to as **Reduced Beam Section (RBS)**. → **Economical solution !!**

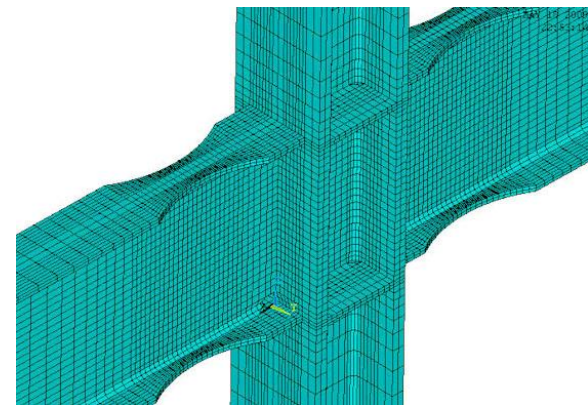
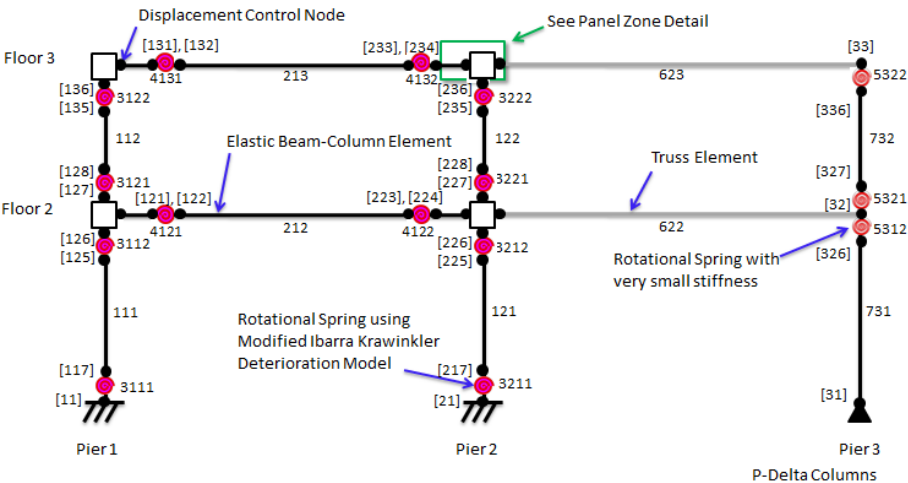
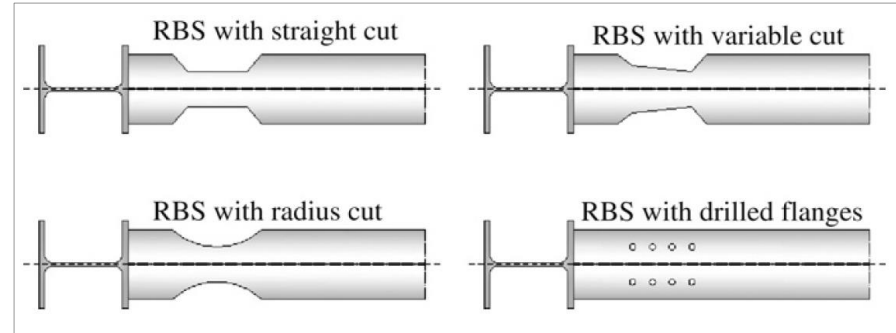
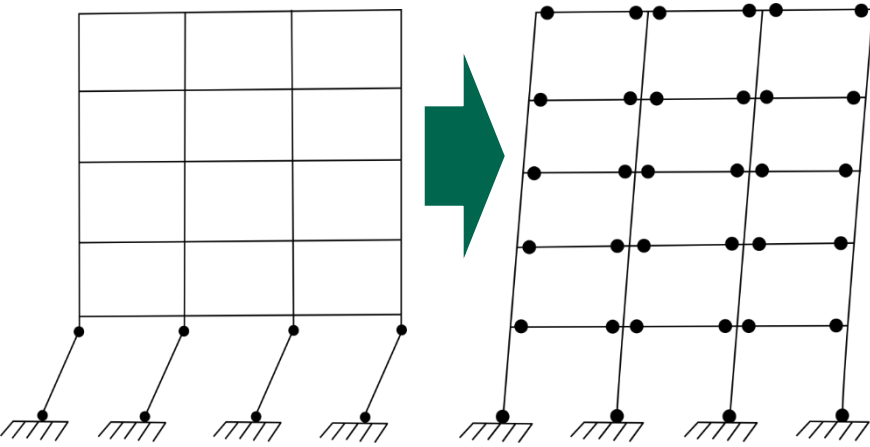






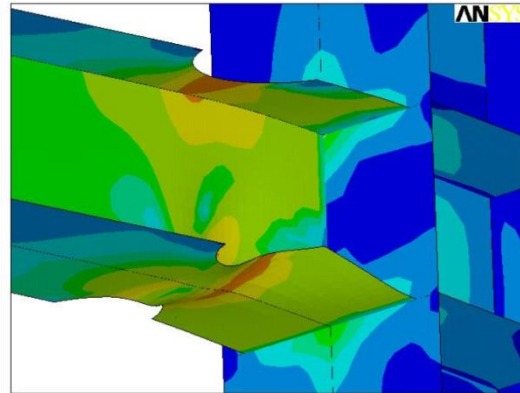
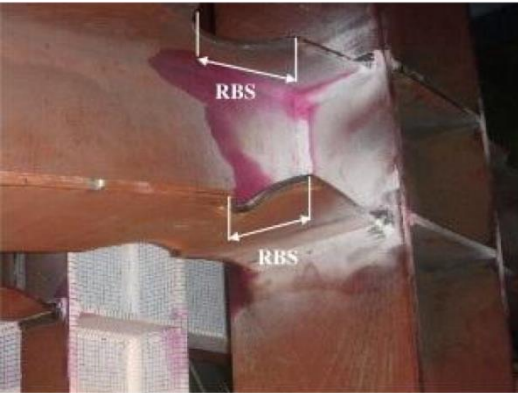
# Modelling of RBS

## Strong column – weak beam





# RBS vs RWS



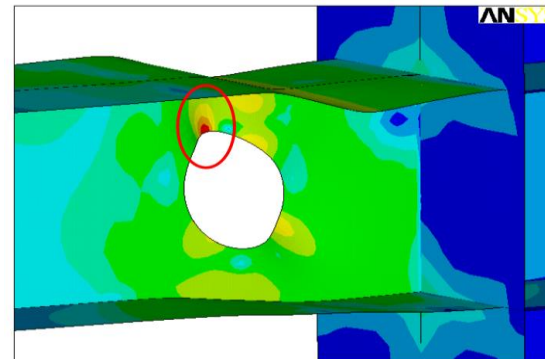
## RBS

- ✓ Increased shear capacity
- ✓ Close to the connection
- ✓ Global instability issues (LTB)
- ✓ Time and cost of rehabilitation can be increased due to the slab

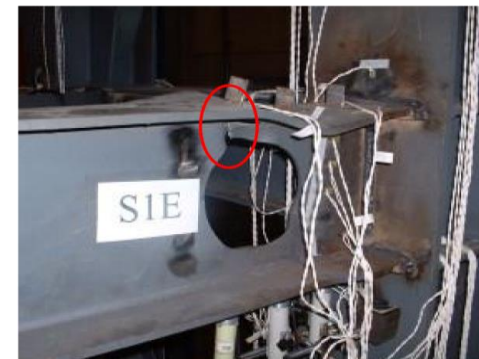
*Pachoumis et al., 2009. Reduced Beam Section Moment Connections, Engineering Structures*

## Reduced Web Section - RWS

- ✓ Easy access to cut during retrofitting (cut-out process)
- ✓ Full shear connection between composite slab-shear studs-top flange (not affected)
- ✓ Easy to inspect & maintain
- ✓ Accommodate services...



*Tsavidaridis et al., 2014. Perforated Steel Beam-to-Column Connections, Journal of Earthquake Engineering*

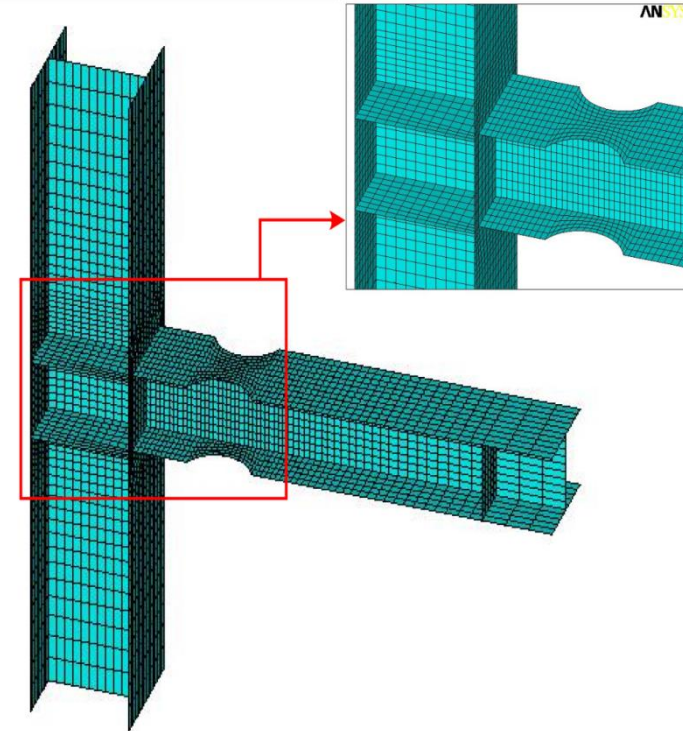
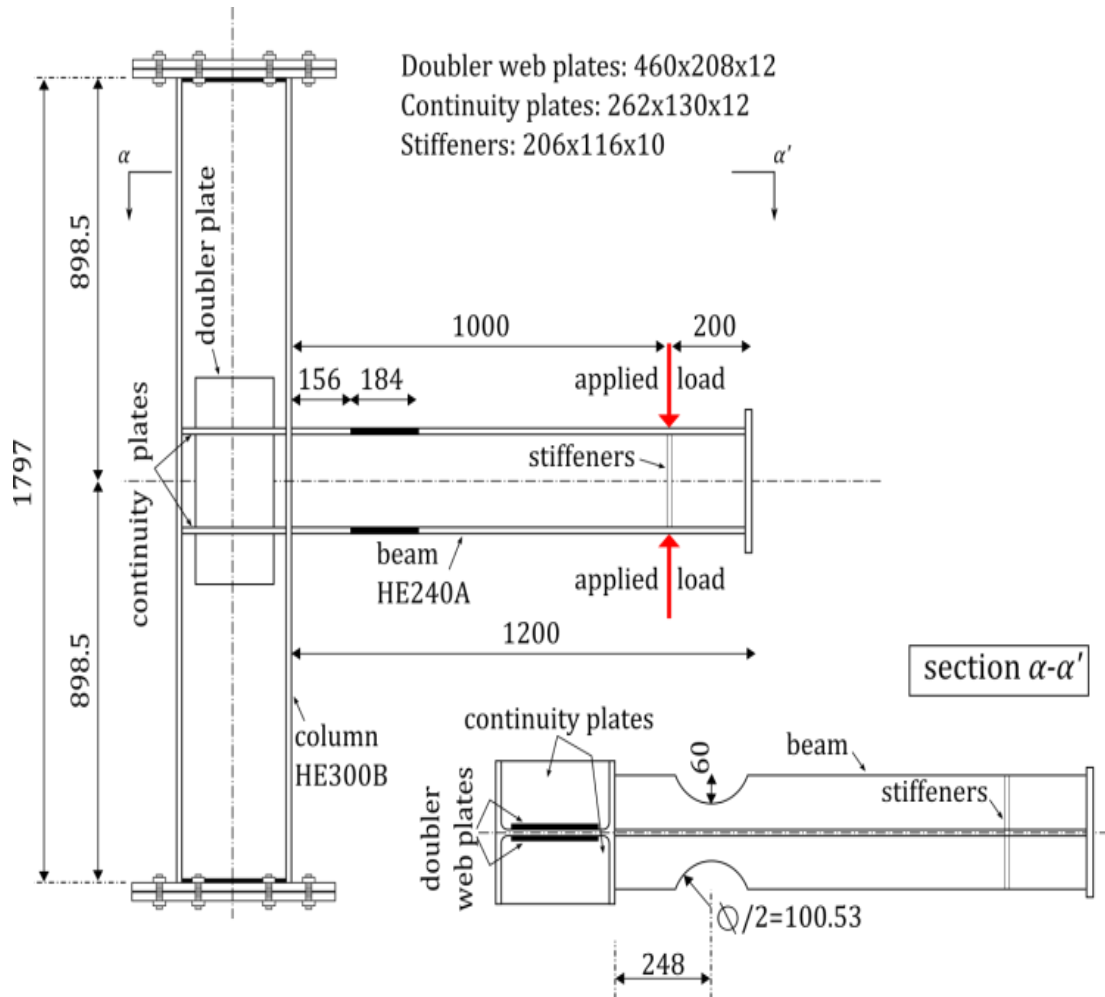


*Yang et al., 2009. Aseismic behaviours of steel MRF with opening in beam web, J. Con. Steel. Res.*



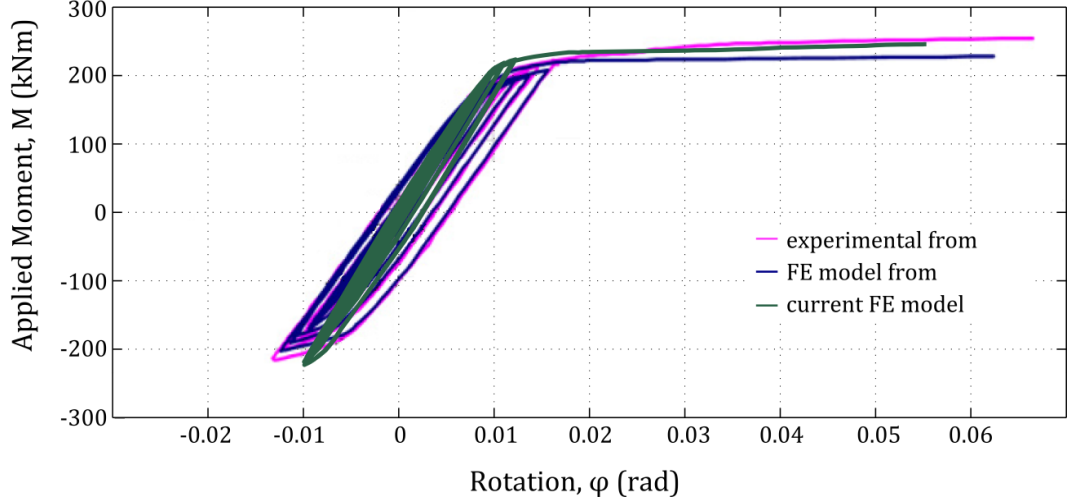
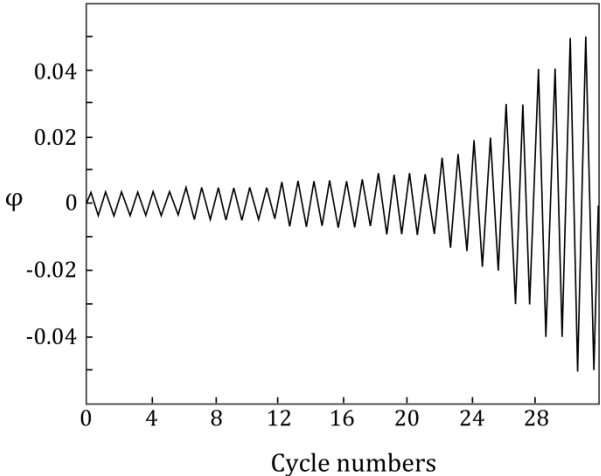
## Fully-Welded Connections with isolated web openings

# Test Apparatus



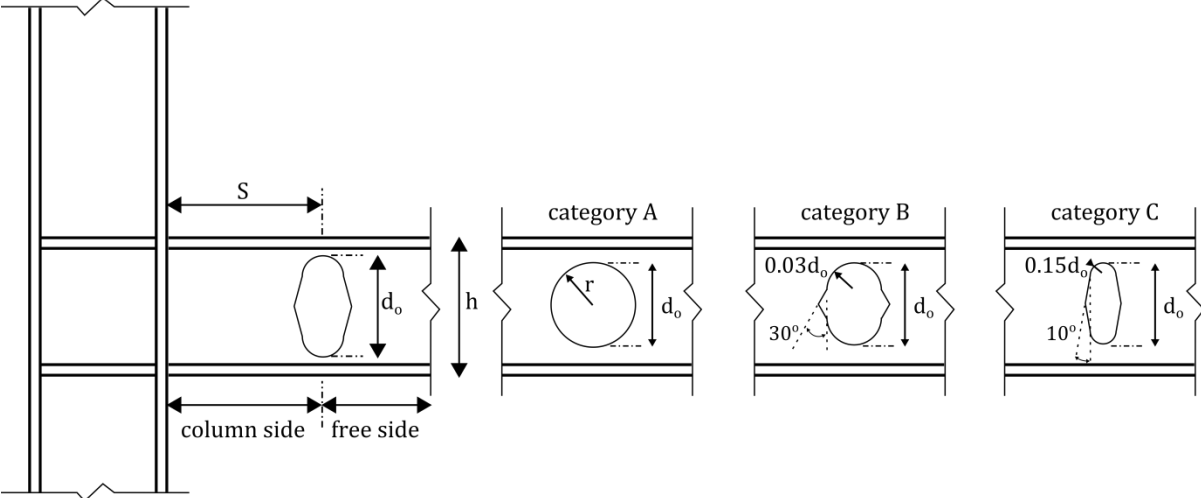
- ✓ 4-node Shell elements
- ✓ Manual (mapped) meshing
- ✓ Mesh convergence
- ✓ Eigen buckling employed

# Parametric Study



## SAC protocol, FEMA 350/AISC

## FE model vs. Experimental

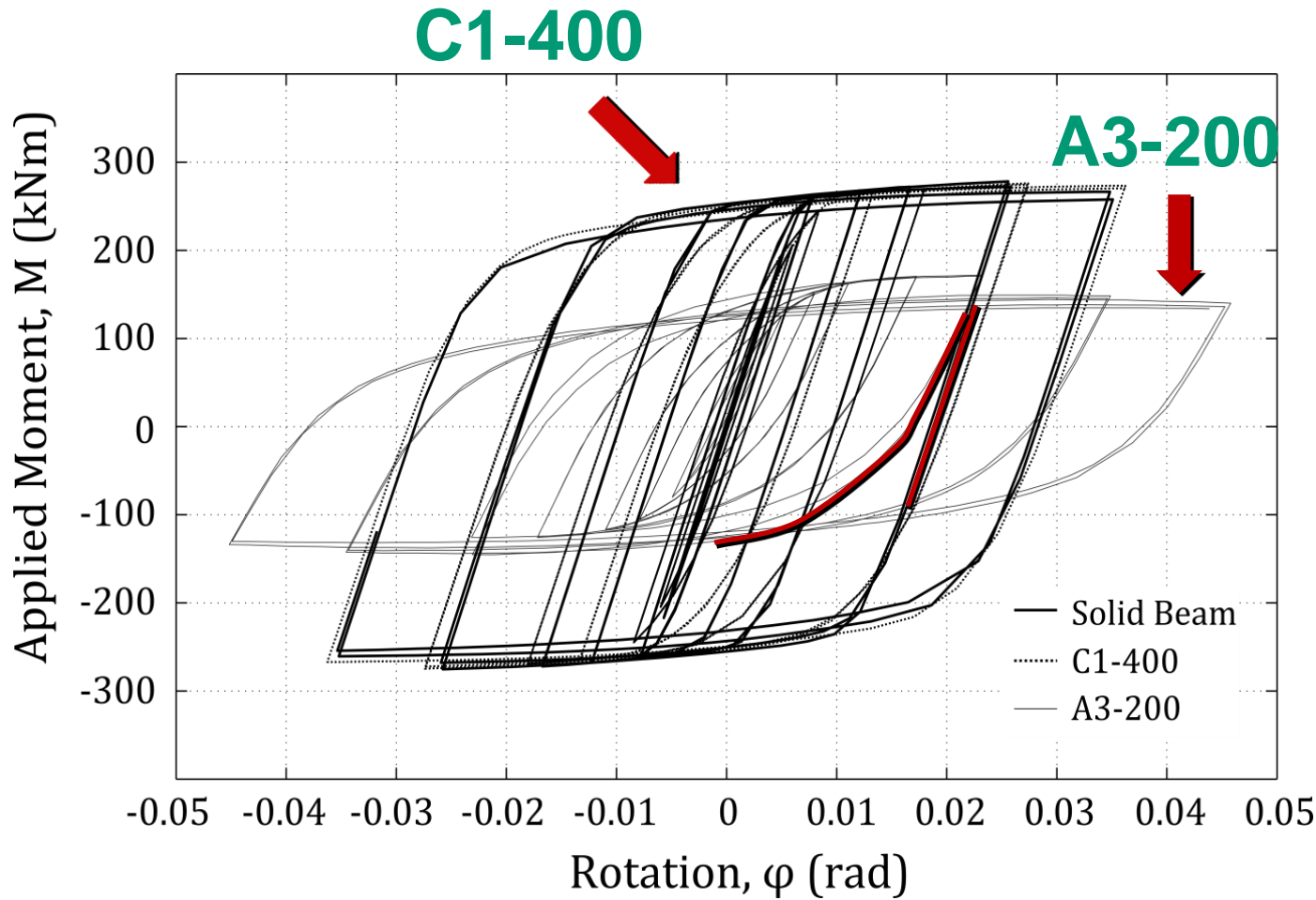


Three digit number representing parameter S;  
(200mm, 300mm, 400mm)

Number indicating the opening depth ( $d_o$ );  
(1:  $d_o=0.5h$ , 2:  $d_o=0.65h$ , 3:  $d_o=0.8h$ )

Letter representing opening's category;  
(A, B, C)

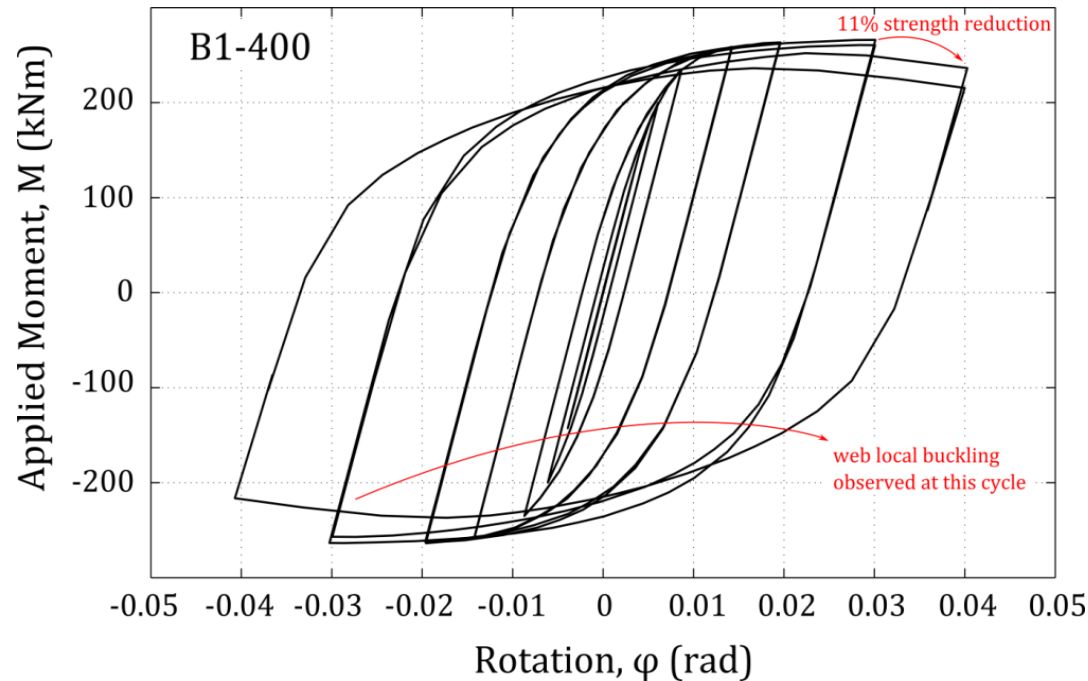
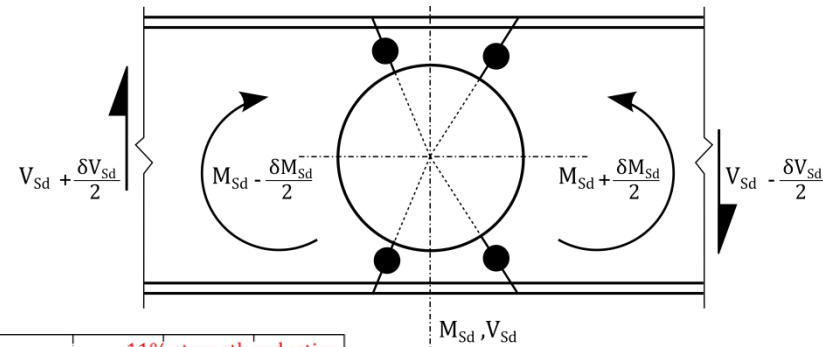
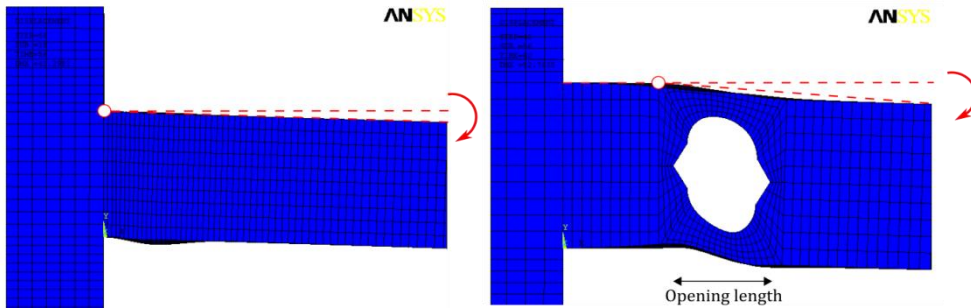






# Rotational Capacity

The higher the critical opening length,  $c$  → the higher is the strength degradation.

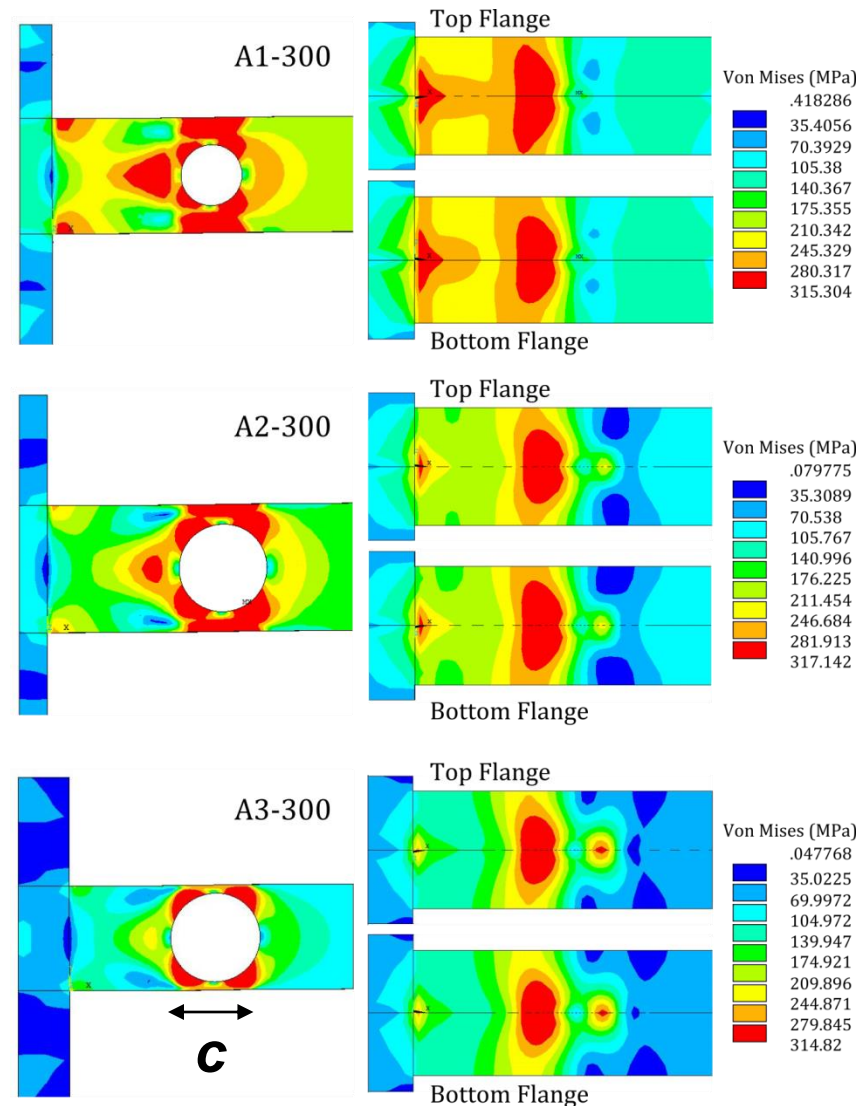


# Vierendeel (Vert. Shear) Mechanism

Vierendeel deformation is introducing **additional rotation to the RWS connections** at the position of the opening, unlike to the connections with solid beams.

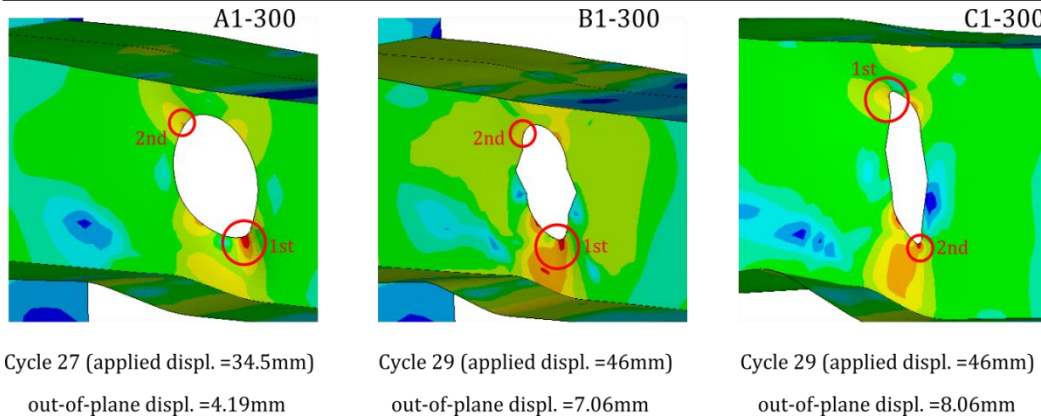
This is the well-known **Vierendeel mechanism** and it is controlled by the **critical opening length,  $c$** .

**Traditional** cellular beams showed a **premature** formation of plastic hinges compared to the **novel perforated beams**.



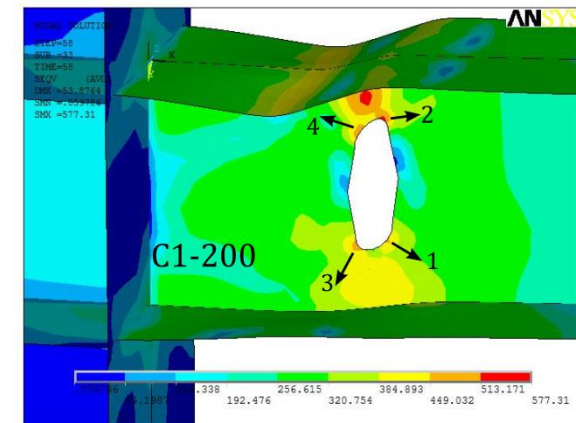
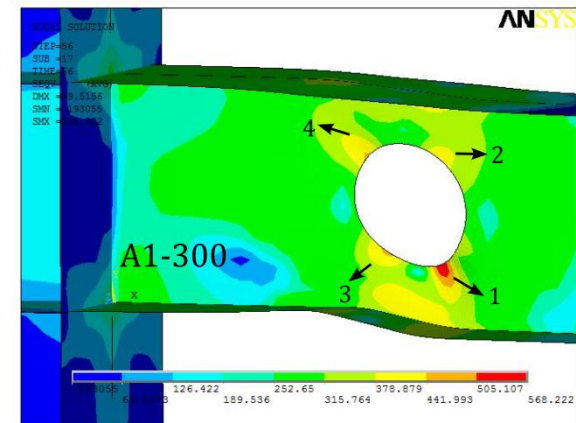


# Position and Movement of Plastic Hinges

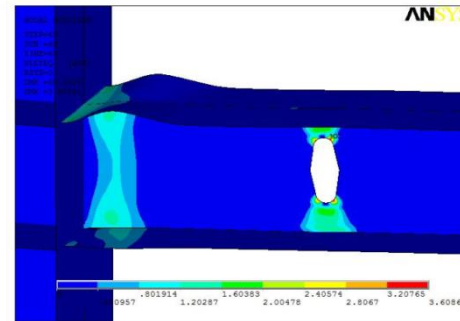
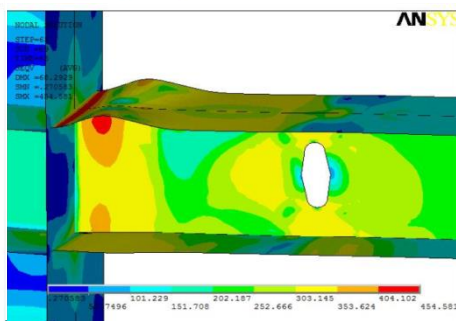
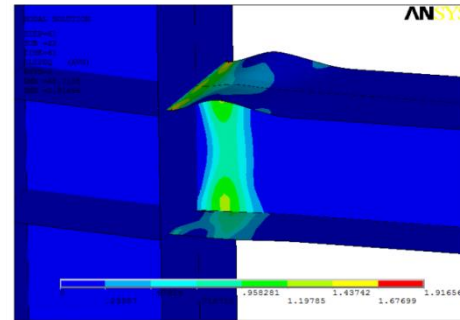
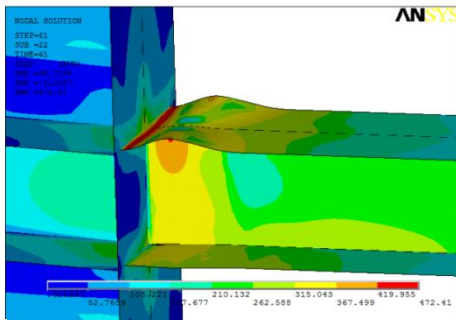


Von-Mises stress contour plot and Plastic hinge formation; left: at cycle 29, right: at cycle 27

## Web local buckling



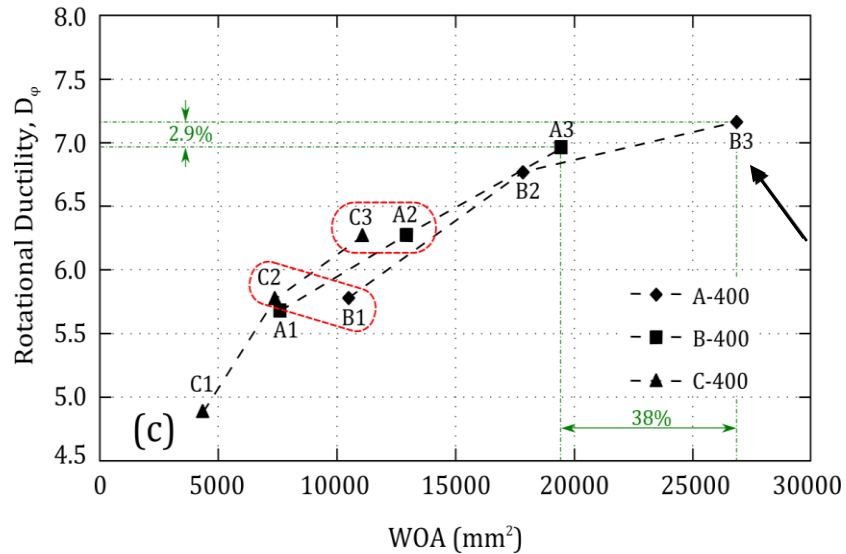
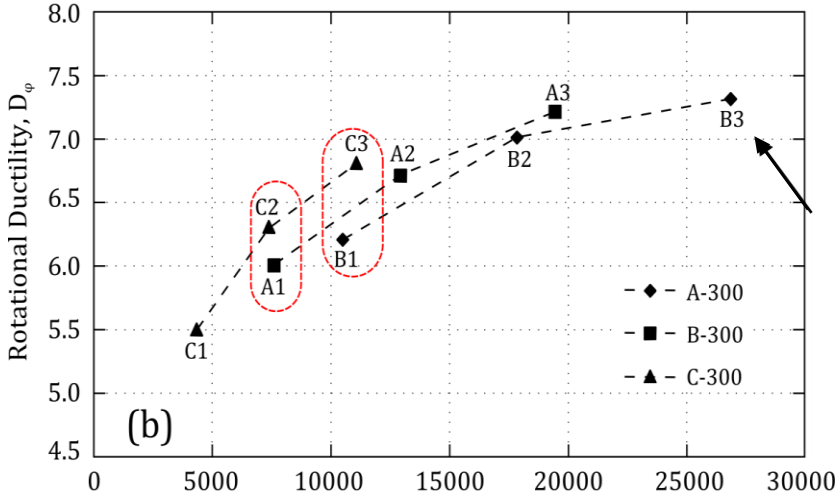
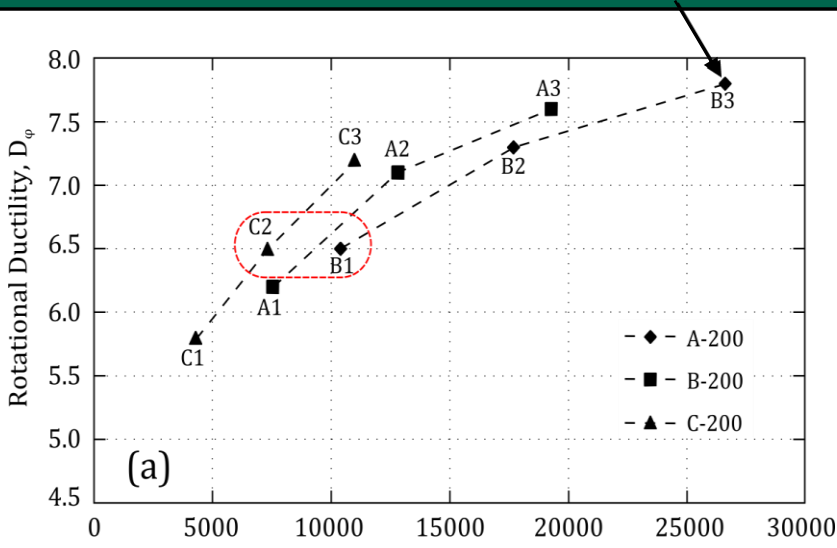
Solid beam



Von-Mises stress (left) and EPEQ (right) contour plots at cycle 31 (loading at 57.5mm)

C1-400

# Effect of WOA on Ductility







# Geometric Parameters

## Parameter do



**Bigger**



**Vierendeel**



**Reducing stress  
from the joint**

## Parameter S



**Smaller**



**Vierendeel**



**Reducing stress  
from the joint**

**Balance**



## Ideal Design



**Large novel B,C or  
Medium circular A**



**Controlled Vierendeel**

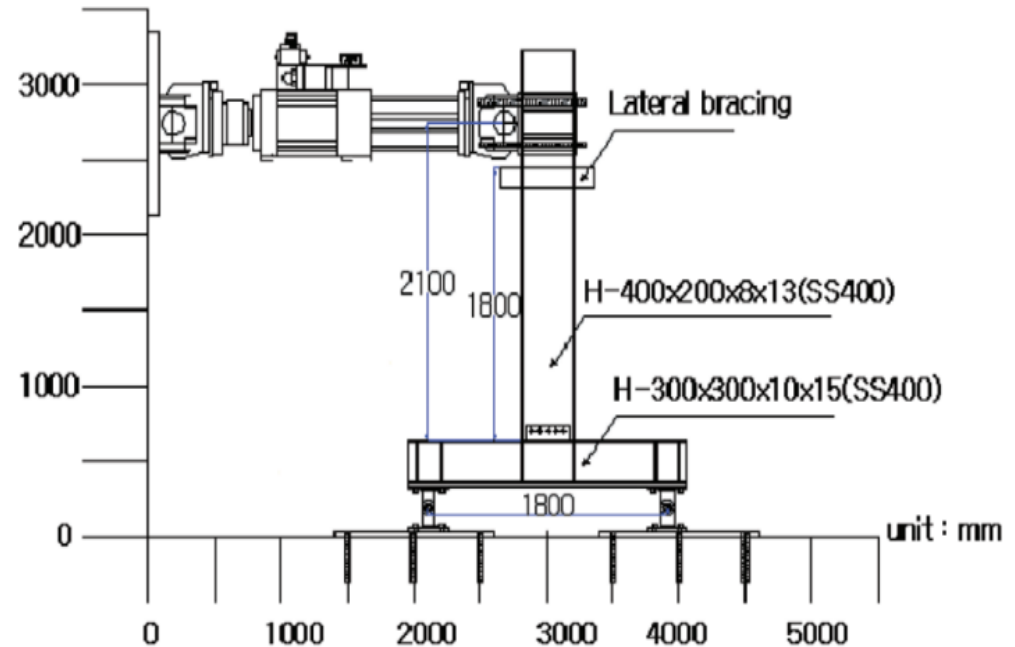
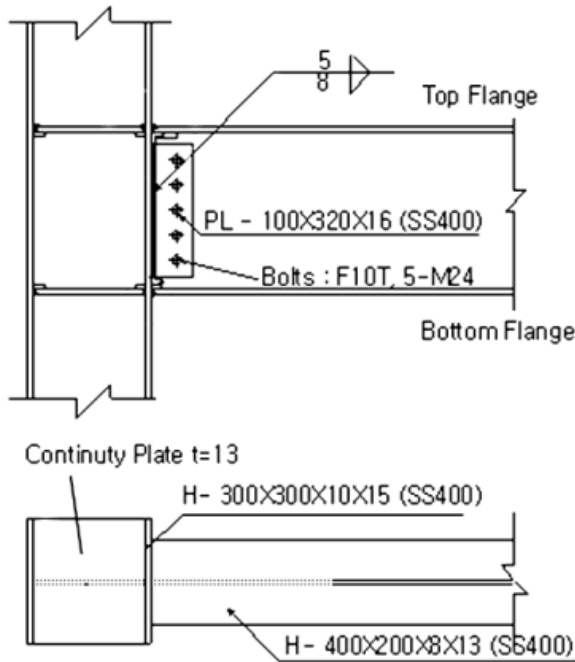


**Controlled Plasticity**

The study presented examined the behaviour of **isolated web openings** in enhancing the ability of a pre-Northridge connection.

- ▲ Introduction of the web openings resulted in some **reduction of strength**.
- ▲ When **small** web openings were used, there was **not any beneficiary effect**.
- ▲ **Isolated large web** openings can prevent excessive shear deformation as well as **reduce the stress intensity** in the vicinity of the beam-to-column weld in contrast to other reinforcing methods (eg. **stiffeners and double plates**).
- ▲ Specimens with **novel** openings, compared to the traditional circular openings, were found to be **stronger** and attain a **higher ultimate rotation** for the same opening depth.
- ▲ **RWS connections achieved higher inelastic rotational capacity in comparison to the RBS connection and they are capable of reaching beyond 0.035 radian [FEMA 350; GSA 2003; DoD 2005].**

## **Welded Unreinforced Flange-Bolted web (WUF-B) Connections with isolated web openings**



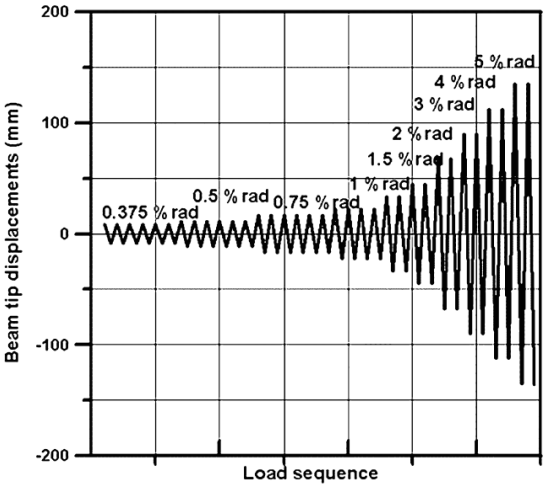
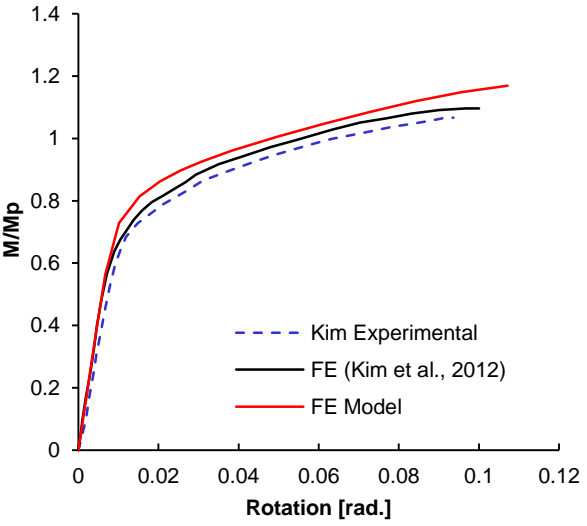
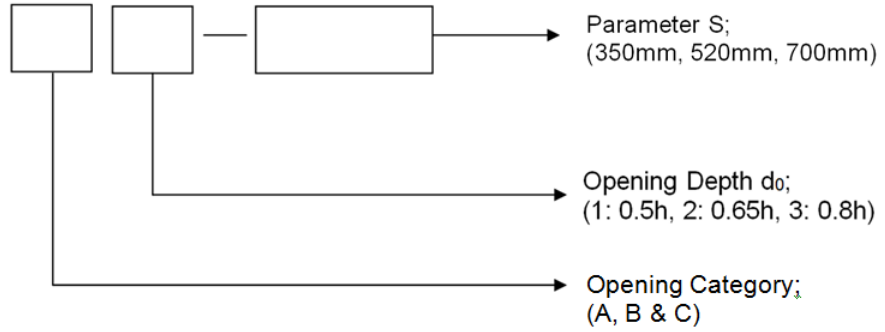
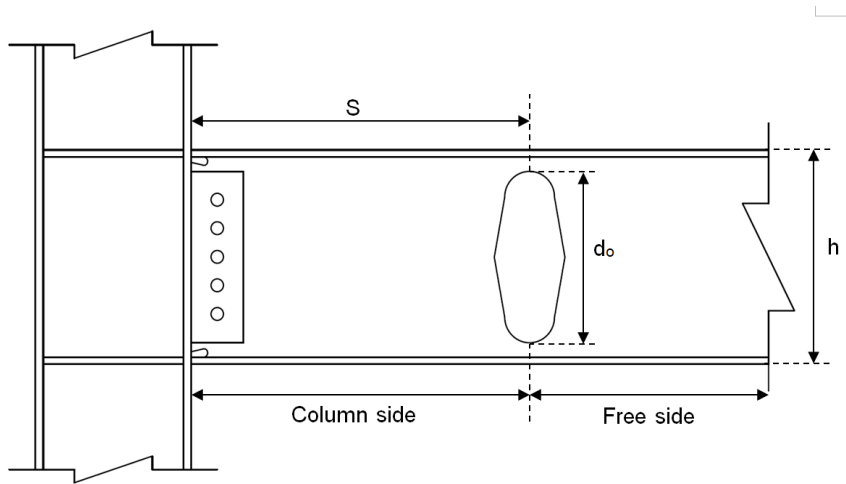
Member	Component	Yield Strength, $f_y$ [MPa]	Ultimate Strength, $f_u$ [MPa]
Beam	Flange	281	423
	Web	332	438
Column	Flange	281	433
	Web	304	450

*Kim et al., 2012. Collapse resistance of unreinforced steel moment connections, The Struct. Design of Tall and Special Build., 21(10), 724-735*

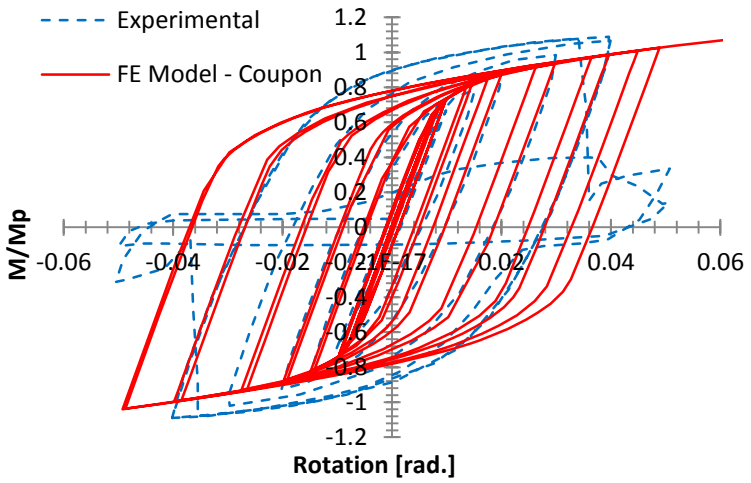
# Parametric Study



UNIVERSITY OF LEEDS

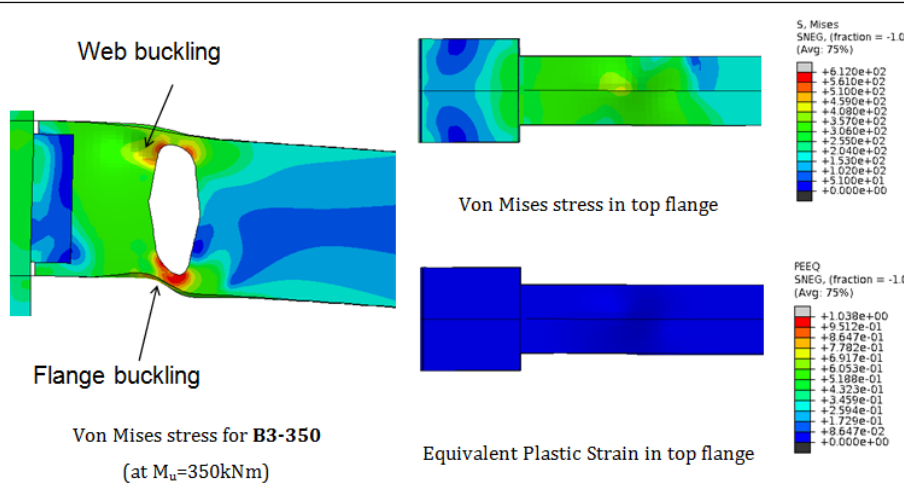
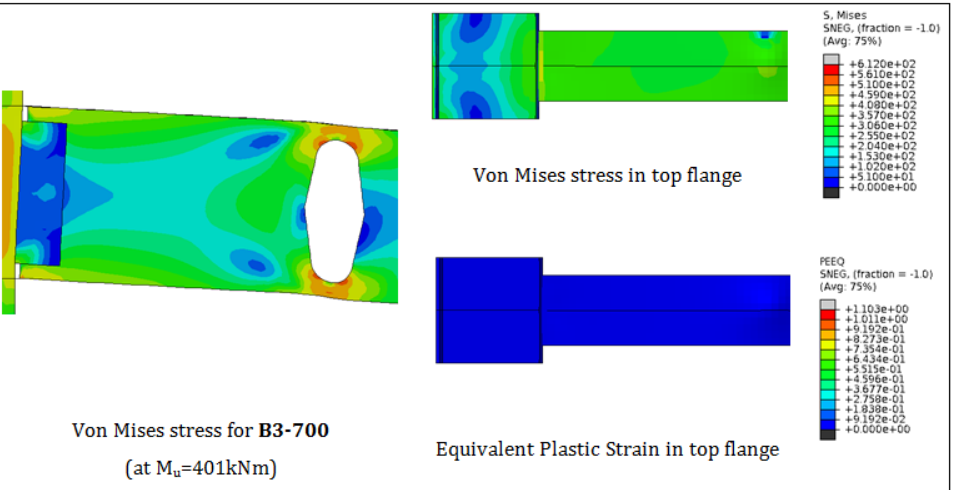
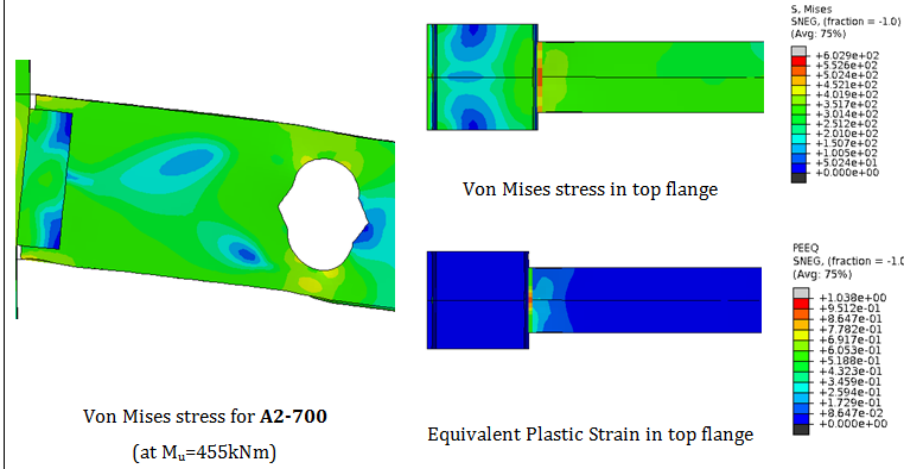
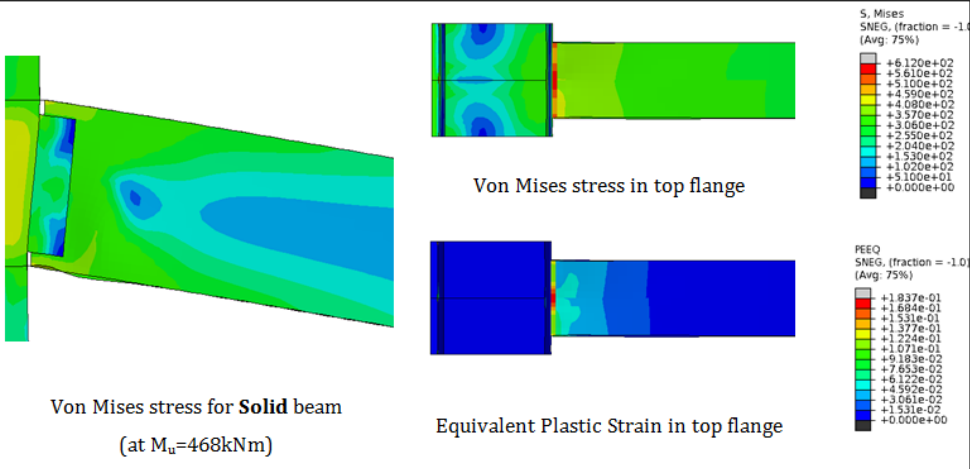


SAC protocol, FEMA/AISC





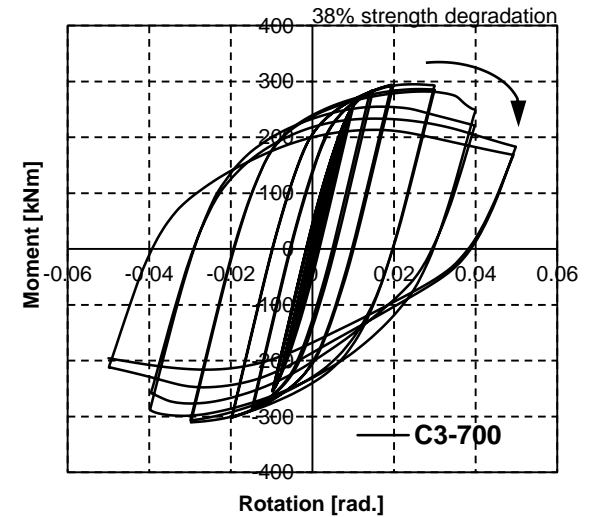
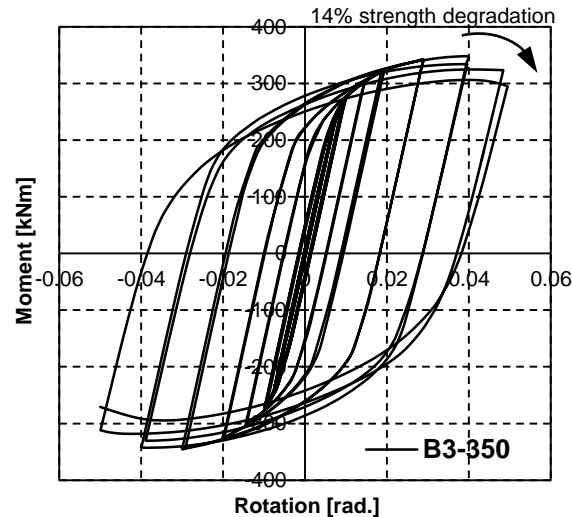
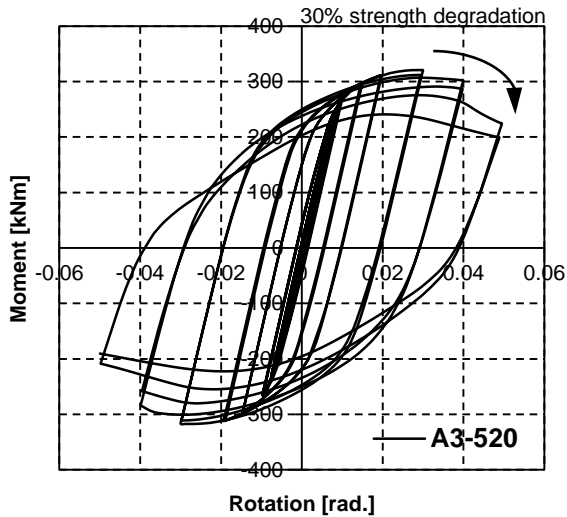
# Vierendeel (Vert. Shear) Mechanism



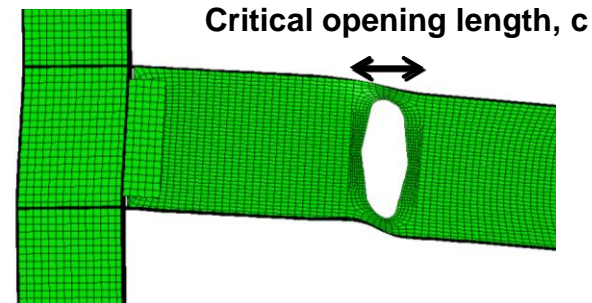
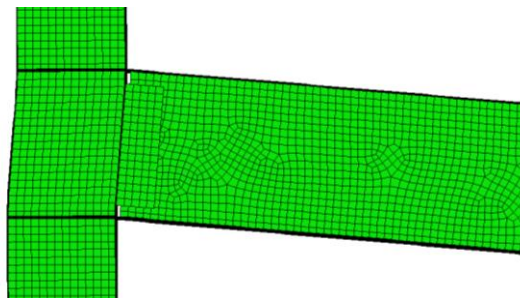


# Rotational Capacity

The higher the critical opening length,  $c \rightarrow$  the higher is the strength degradation.

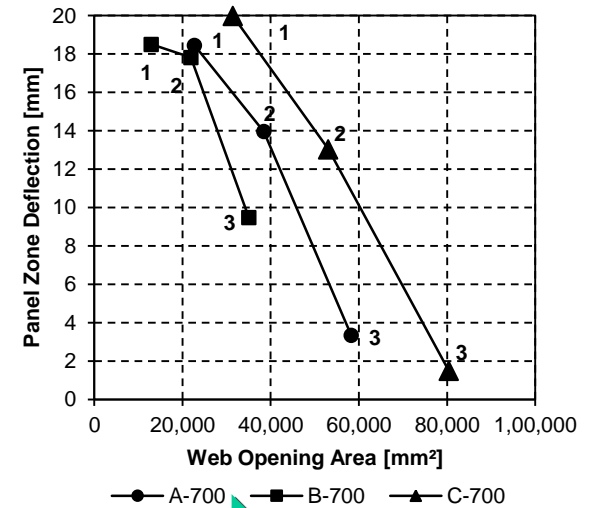
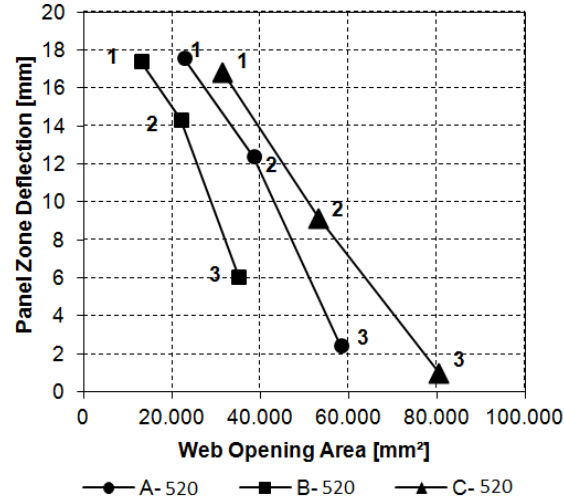
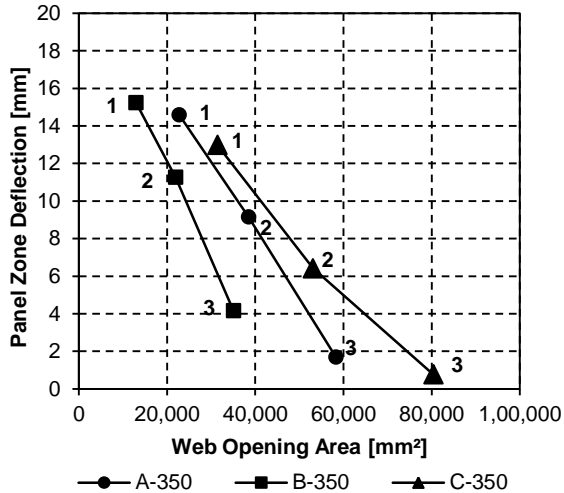


Centre of rotation for solid and perforated beams.

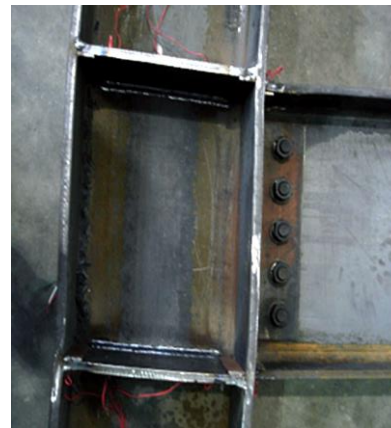
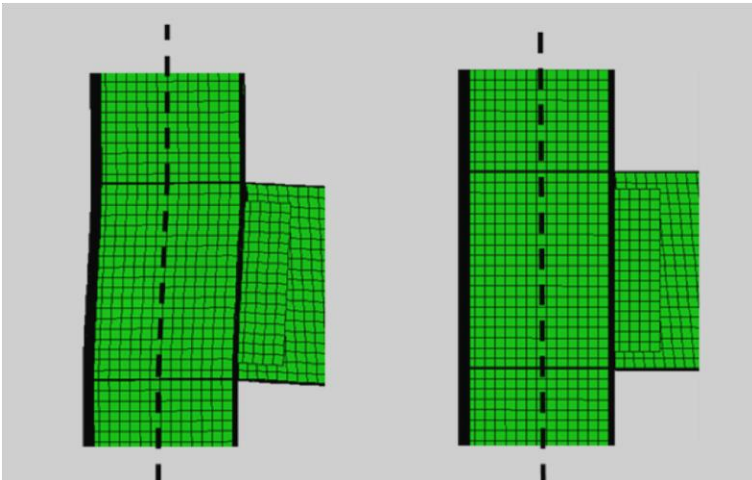




# Panel Zone Deformation



S increases



Medium and Large web openings placed near the connection.



Cheaper solution instead of using stiffeners...

Relative panel zone deformation from column centreline

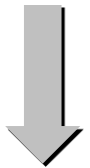


# Geometric Parameters

Parameter do



Bigger



Vierendeel

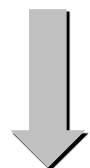


Reducing stress  
from the joint

Parameter S



Smaller



Vierendeel



Reducing stress  
from the joint

Balance



Ideal Design



Large novel B or  
Medium A, C



Controlled Vierendeel



Controlled Plasticity



# Further Concluding Remarks

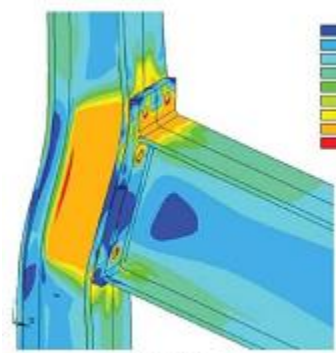
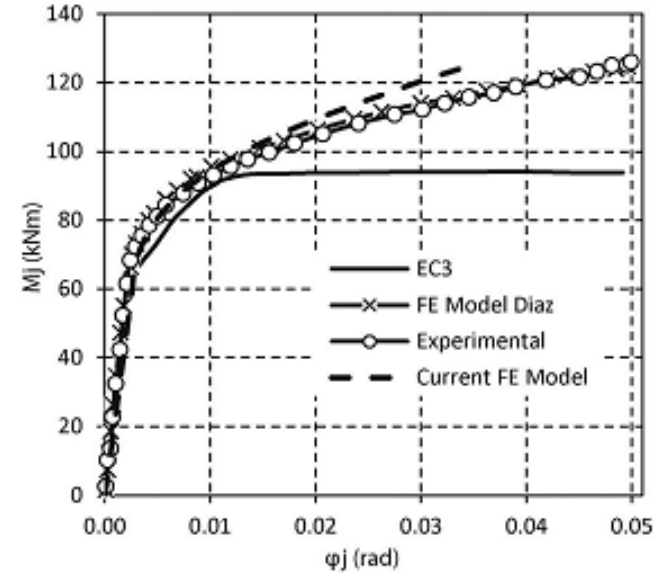
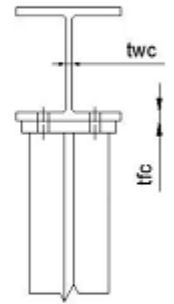
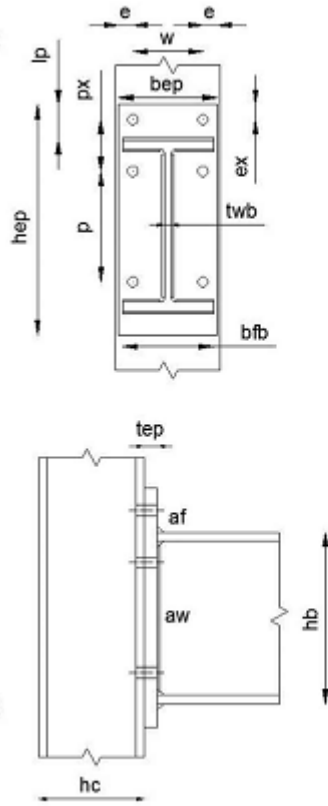
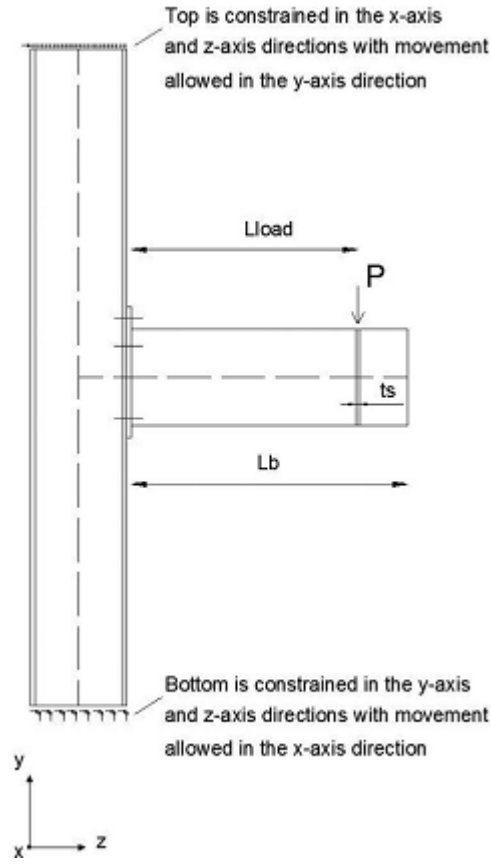
- ▲ The use of **large** openings was found to be the most effective at moving the plastic region away from the column's face and the bolts, while **reducing the shear zone panel deflection**.
- ▲ The most effective design was considered when a large **narrow elliptically-based web opening** was employed.
- ▲ The use of such narrow novel openings is **ideal** as the connection is **not that sensitive to geometric changes**.
- ▲ The actual **position, shape and size** of the perforation is of critical importance to the effective ductile design.



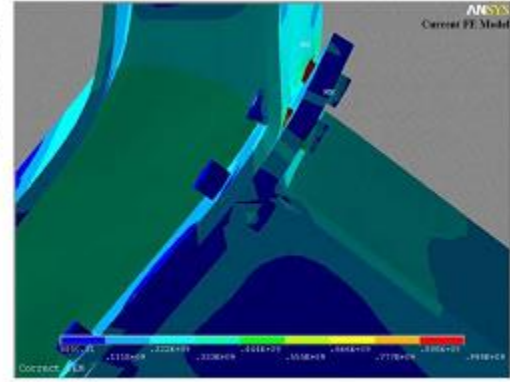
## **Extended End-plate Bolted Connections with isolated as well as periodically spaced circular web openings**



# Test Apparatus



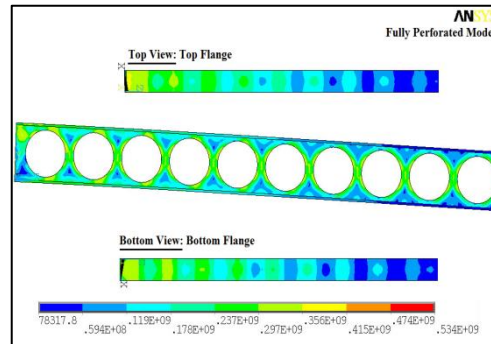
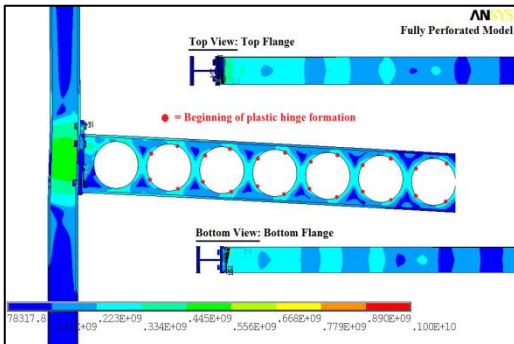
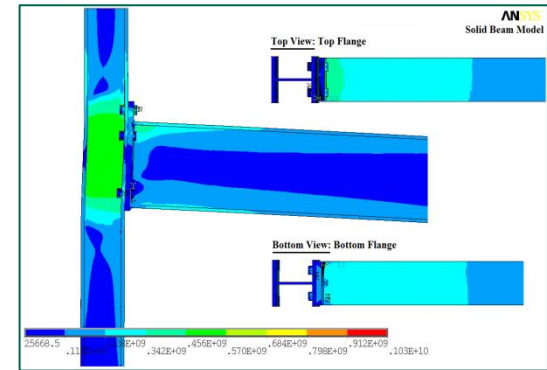
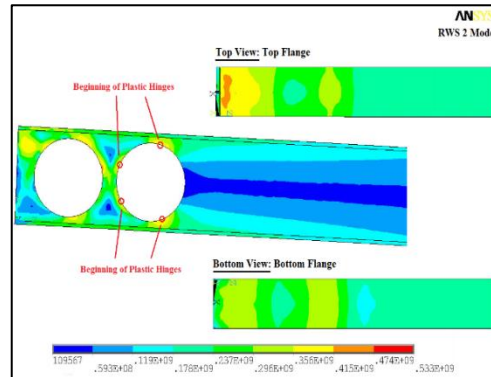
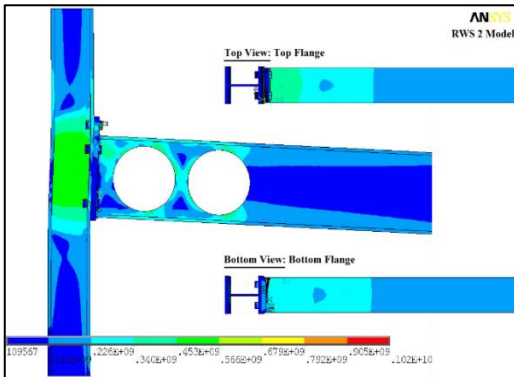
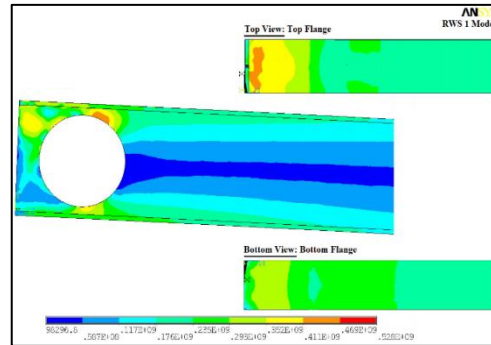
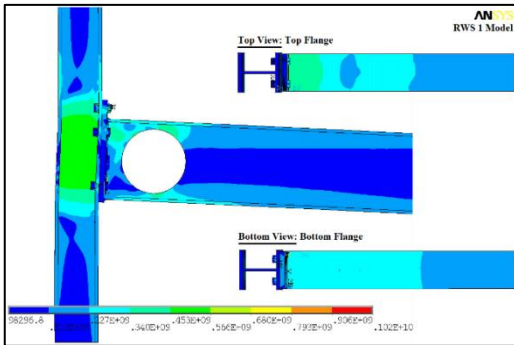
(a)  $M_{j,u} = 124.1\text{kNm}$



(b)  $M_{j,u} = 124.1\text{kNm}$



# Parametric Study on Circular Openings



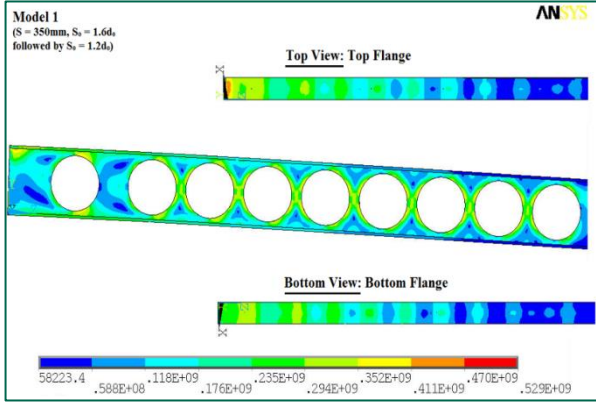
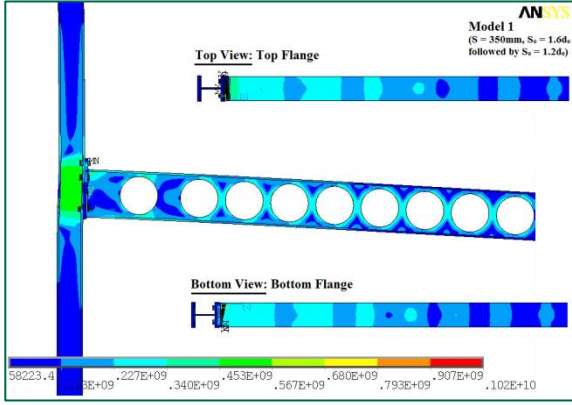
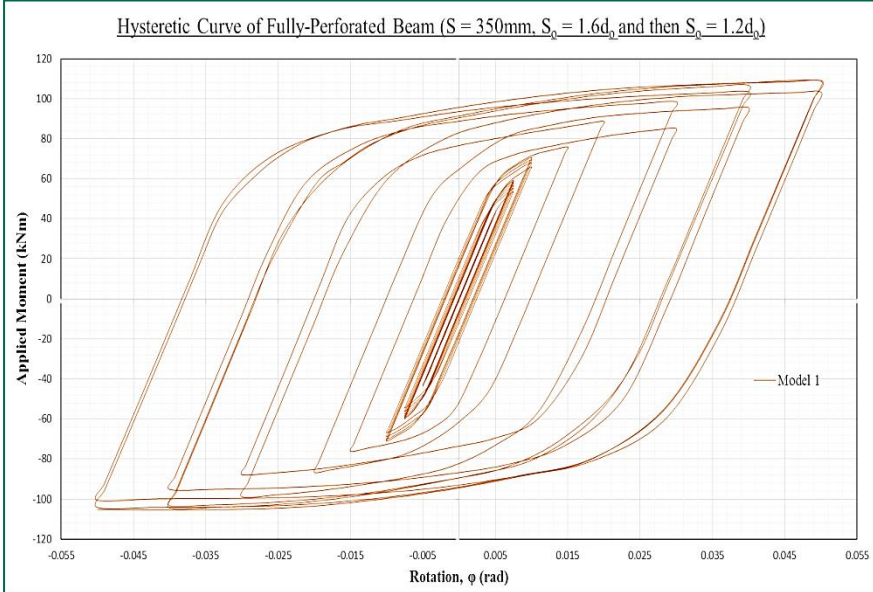
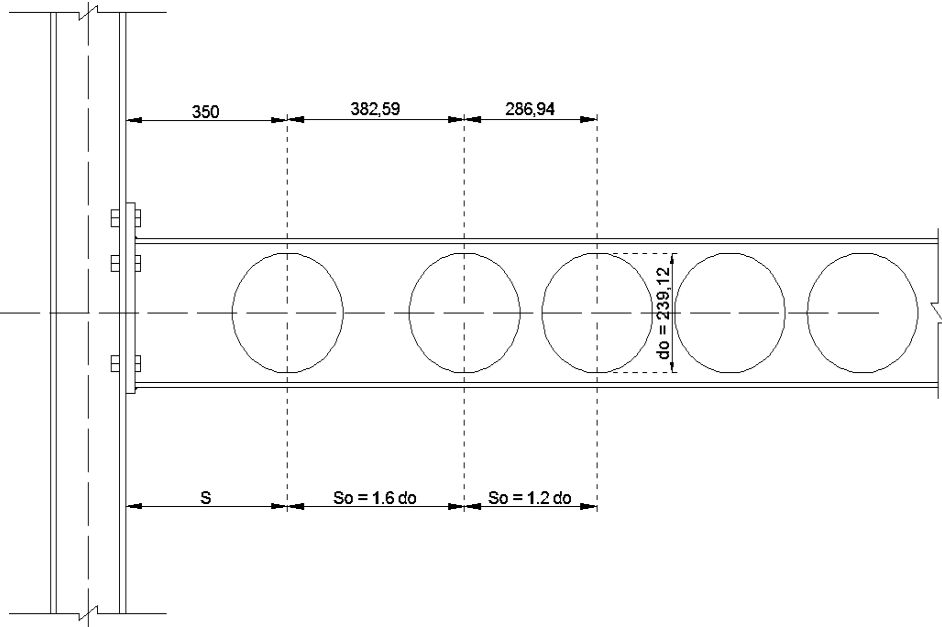
Specimen	Number of Circular Perforations	Yield Moment $M_y$ (kNm)	Ultimate Moment $M_u$ (kNm)	Yield Rotation $\phi_y$ (rad)	Ultimate Rotation $\phi_u$ (rad)
Solid Beam	-	74.59	128.58	0.009592	0.049989
RWS 1	1	71.14	112.19	0.009492	0.050025
RWS 2	2	64.41	117.23	0.009292	0.050025
Fully Perforated	10	62.44	105.77	0.008555	0.050004

Specimen	Rotational Ductility $D_\phi$	Initial Rotational Stiffness $K_i$ (kNm/rad)	Web Opening Area (mm <sup>2</sup> )	Dissipated Energy $E$ (kNm)(rad)
Solid Beam	5.21	10329.89	-	107.14
RWS 1	5.27	10240.98	44907.8	92.50
RWS 2	5.38	9167.77	44907.8	81.19
Fully Perforated	5.85	8097.76	44907.8	45.94

# Additional Innovative Designs

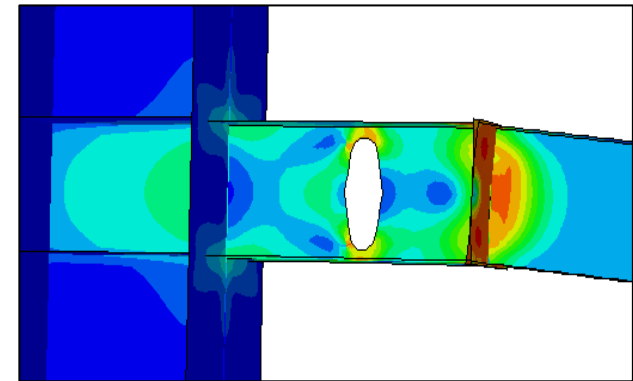
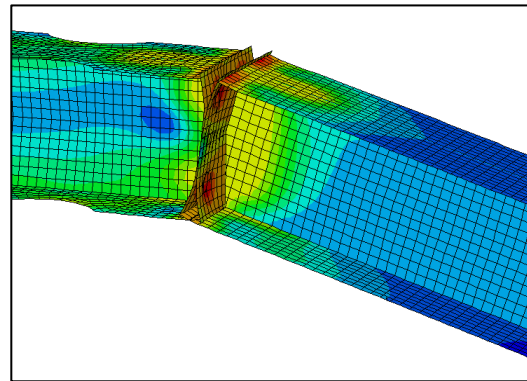
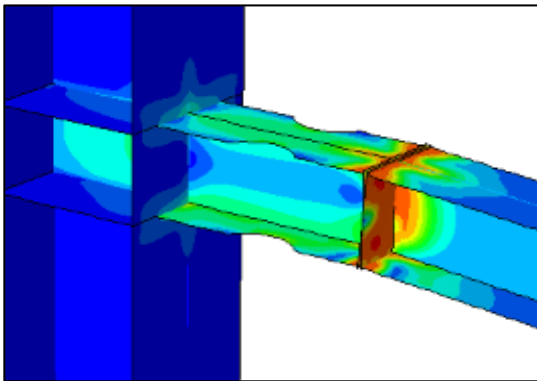


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# Further Concluding Remarks

- ▲ The **position of the first** from the support web opening is again critical.
- ▲ Providing there are **not web openings** close to the supports, there is no mobilisation of the high stresses from the shear panel zone.
- ▲ The concept of the “**fuse mechanism**” with controlled damage and replacement can be realised.



- Completing the experimental campaign on RWS will lead us to the incorporation of such connections to EC8 and other design guidelines.



# Aknowledgments (funding & technical support)



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**ASD metal services**

klöckner & co multi metal distribution



Structural**Awards**2013  
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**EPSRC**

Engineering and Physical Sciences  
Research Council



Tsavdaridis, K.D. and Pilbin, C. FE Parametric Study of WUF-B Moment Connections with Single Mixed Shape Beam Web Openings under Monotonic and Cyclic Loading. **International Journal of Steel Structures**. 2016

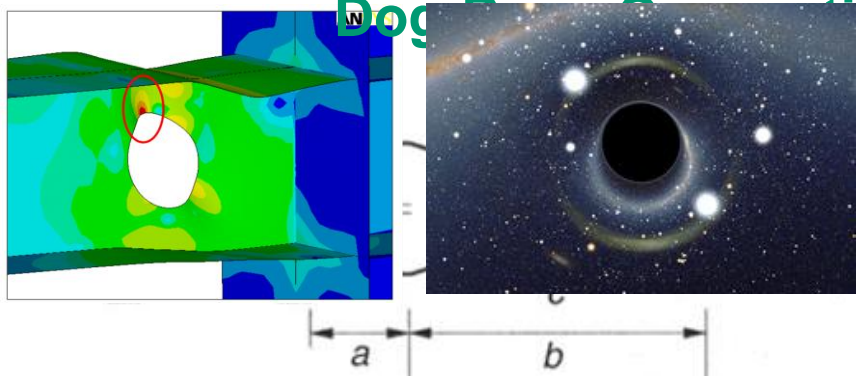
Tsavdaridis, K.D. and Papadopoulos, T. A FE Parametric Study of RWS Beam-to-Column Bolted Connections with Cellular Beams. **Journal of Constructional Steel Research**, Vol. 116, 2016, pp. 92-113

Tsavdaridis, K.D. Strengthening Techniques: Code-Deficient Steel Buildings, to the section: Structural Engineering - Structural Design, **Encyclopedia of Earthquake Engineering**, Springer Verlag, 2014, pp. 1-26

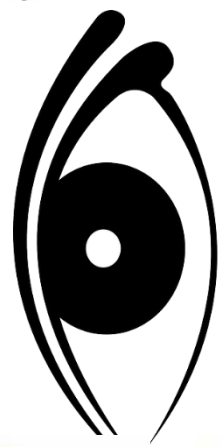
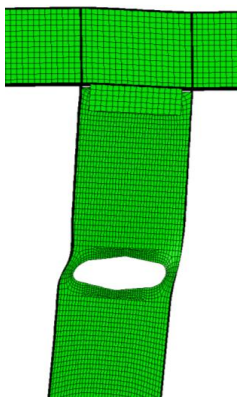
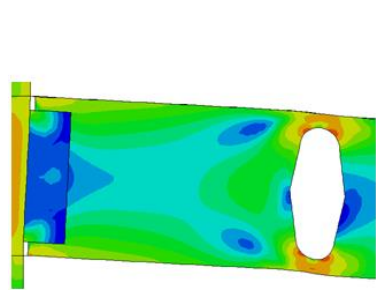
Tsavdaridis, K.D., Faghih, F. and Nikitas, N. Assessment of Perforated Steel Beam-to-Column Connections Subjected to Cyclic Loading. **Journal of Earthquake Engineering**, Vol. 18, Issue 8, 2014, pp. 1302-1325



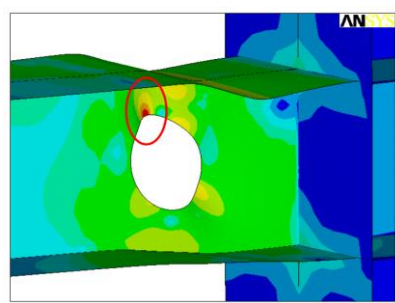
## Dog Bone Connection



*Black (White) Hole Connection?*



*Eye connection?*



*Donut Connection?*